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Environment Engineering

CIVIL ENGINEERING

Date of Test : 10/06/2025

ANSWER KEY ➤

| | | | | |
|--------|---------|---------|---------|---------|
| 1. (b) | 7. (b) | 13. (d) | 19. (c) | 25. (d) |
| 2. (a) | 8. (d) | 14. (a) | 20. (d) | 26. (a) |
| 3. (b) | 9. (b) | 15. (b) | 21. (d) | 27. (d) |
| 4. (a) | 10. (d) | 16. (d) | 22. (b) | 28. (c) |
| 5. (d) | 11. (a) | 17. (c) | 23. (c) | 29. (b) |
| 6. (a) | 12. (c) | 18. (b) | 24. (b) | 30. (a) |

DETAILED EXPLANATIONS

1. (b)
⇒

$$\begin{aligned} TH &= \text{Ca}^{2+} \text{ as CaCO}_3 + \text{Mg}^{2+} \text{ as CaCO}_3 \\ &= \frac{120}{20} \times 50 + \frac{48}{12} \times 50 = 500 \text{ mg/l as CaCO}_3 \\ \text{Alkalinity} &= [\text{HCO}_3^-] \text{ as CaCO}_3 \\ &= \frac{183}{61} \times 50 = 150 \text{ mg/l as CaCO}_3 \\ CH &= \min(TH, \text{Alkalinity}) = 150 \text{ mg/l} \\ NCH &= TH - CH = 350 \text{ mg/l} \end{aligned}$$

2. (a)

$$\begin{aligned} \text{Total organic loading rate} &= 300 \times (1 - 0.3) \times 6 \text{ kg/day} \\ y &= 1260 \text{ kg/day} \\ F &= 2 \\ V &= 2000 \text{ m}^3 = 0.2 \text{ ha-m} \\ \eta &= \frac{100}{1 + 0.0044 \sqrt{\frac{y}{VF}}} = \frac{100}{1 + 0.0044 \sqrt{\frac{1260}{2 \times 0.2}}} \\ \eta &= 80.2\% \end{aligned}$$

3. (b)

$$\begin{aligned} Q &= 10 \text{ m}^3/\text{sec} \\ v_d &= 2 \times 10^5 \times (1 \times 10^{-6}) = 0.2 \text{ m/sec} \\ \eta &= 1 - e^{-\left(\frac{Av_d}{Q}\right)} \\ 0.90 &= 1 - e^{-\left(\frac{Av_d}{Q}\right)} \\ e^{-\left(\frac{Av_d}{Q}\right)} &= 0.1 \\ -\frac{AV_d}{Q} &= \ln(0.1) = -\ln 10 = -2.303 \\ \Rightarrow A &= 115.13 \text{ m}^2 \end{aligned}$$

4. (a)

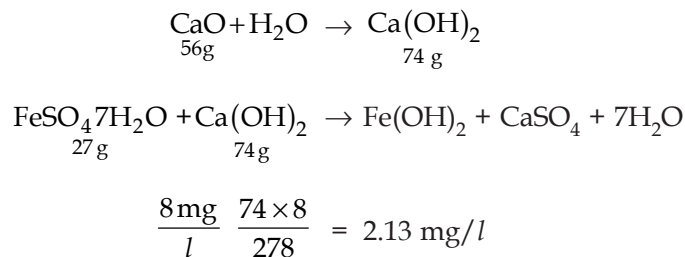
$$\begin{aligned} Q &= \frac{2\pi T(S_1 - S_2)}{\ln\left(\frac{r_2}{r_1}\right)} \\ T &= \frac{Q \ln\left(\frac{r_2}{r_1}\right)}{2\pi(S_1 - S_2)} = \frac{5000 \times 24 \times 60 \times 10^{-3} \ln\left(\frac{200}{20}\right)}{2\pi(4 - 1)} \\ T &= 879.5 \text{ m}^2/\text{day} \simeq 880 \text{ m}^2/\text{day} \end{aligned}$$

5. (d)
Catabolism is destructive phase of metabolism.

6. (a)

$$\begin{aligned}
 &\Rightarrow [pH]_A = 5.5 \\
 &[H^+]_A = 10^{-5.5} \text{ and } [OH^-]_A = 10^{-(14-5.5)} \\
 &[OH^-]_A = 10^{-8.5} \\
 &[OH^-]_B = 20[OH^-]_A = 20 \times 10^{-8.5} = 2 \times 10^{-7.5} \\
 &\Rightarrow [H^+]_B = \frac{10^{-14}}{2 \times 10^{-7.5}} = \frac{10^{-6.5}}{2} \\
 &[pH]_B = -\log \left[\frac{10^{-6.5}}{2} \right] \\
 &= 6.5 + \log 2 \\
 &= 6.5 + 0.3 \\
 &[pH]_B = 6.8
 \end{aligned}$$

7. (b)



Quantity of quicklime for 592 mg/l of Ca(OH)_2 .

$$= \frac{56}{74} \times 2.13 \text{ mg/l} = 1.612 \text{ mg/l}$$

$$\text{For } 10 \times 10^6 \text{ l of water} = 16.12 \text{ kg}$$

8. (d)

$$\begin{aligned}
 \text{SVI} &= \frac{V_{ob} (\text{ml/l})}{X_{ob} (\text{mg/l})} = \frac{V_{ob}}{X_{ob}} \times 100 \text{ ml/g} \\
 V_{ob} &= \text{Settled volume sludge per liter} \\
 &= \frac{850}{2} = 425 \text{ ml/l} \\
 V_{ob} &= 3000 \text{ mg/l} \\
 \text{SVI} &= \frac{425}{3000} \times 1000 = 141.667 \simeq 142 \text{ ml/g}
 \end{aligned}$$

9. (b)

$$\begin{aligned}
 S &= \frac{Q_S S_S + Q_R S_R}{Q_S + Q_R} \\
 \Rightarrow 20 &= \frac{10(350) + Q_R(5)}{10 + Q_R}
 \end{aligned}$$

$$\Rightarrow Q_R = 220 \text{ MLD}$$

$$\Rightarrow Q_R = \frac{220 \times 10^6}{86400}$$

$$\Rightarrow Q_R = 2546.30 \text{ lit/sec} \simeq 2546 \text{ lit/sec}$$

10. (d)

Statement I is false but II is true.

Coliforms are rod-shaped non-pathogenic bacteria.

11. (a)

$$k = \frac{[H^+][CO_3^{2-}]}{[HCO_3^-]}$$

$$[H^+] = 10^{-10} \text{ M}$$

$$[CO_3^{2-}] = \frac{30 \times 10^{-3}}{60} \text{ M} = 5 \times 10^{-4} \text{ M}$$

$$5 \times 10^{-11} = \frac{10^{-10} \times 5 \times 10^{-4}}{[HCO_3^-]}$$

$$[HCO_3^-] = 10^{-3} \text{ M}$$

$$[HCO_3^-] \text{ in mg/l} = 61 \times 10^{-3} \times 10^3 = 61 \text{ mg/l}$$

$$\text{Bi-carbonate alkalinity} = \frac{61}{61} \times 50 = 50 \text{ mg/l as CaCO}_3$$

12. (c)

$$\text{DO of mix} = \frac{9.2 \times 2 + 0.3 \times 0}{2 + 0.3} = 8 \text{ mg/l}$$

$$D_0 = 9.2 - 8 = 1.2 \text{ mg/l}$$

$$\text{BOD}_u = 1.25 \text{ BOD}_5 = 250 \text{ mg/l}$$

$$F = 3$$

$$L = \frac{0 \times 2 + 0.3 \times 250}{2 + 0.3}$$

$$L = 32.609 \text{ mg/l}$$

$$\left(\frac{L}{D_c F} \right)^{f-1} = f \left[1 - (f-1) \frac{D_0}{L} \right]$$

$$\left[\frac{32.609}{D_c \times 3} \right]^2 = 3 \left[1 - \frac{2 \times 1.2}{32.609} \right]$$

$$D_c = 6.52 \text{ mg/l}$$

$$\text{Minimum DO} = 9.2 - 6.52 = 2.68 \text{ mg/l} \simeq 2.7 \text{ mg/l}$$

13. (d)

$$\begin{aligned}
 r &= 15\% \\
 r' &= 2\% \\
 P_{2021} &= 2,00,0000 \\
 P_{2031} &= P_{2021} + \left(\frac{r - r'}{100} \right) \times P_{2021} \\
 &= 200000 + \left(\frac{15 - 2}{100} \right) \times 200000 = 2,26,000 \\
 P_{2041} &= P_{2031} + \left(\frac{r - 2r'}{100} \right) P_{2031} \\
 &= 226000 + \left(\frac{15 - 4}{100} \right) \times 226000 \\
 P_{2041} &= 250860
 \end{aligned}$$

14. (a)

Grit chambers are mainly constructed to prevent the accumulation of inorganic particles in sludge digesters.

15. (b)

$$k = \frac{[\text{HOCl}]}{[\text{H}^+][\text{OCl}^-]}$$

Here concentration is in moles/litre

$$10^{7.4} = \frac{[\text{HOCl}]}{[10^{-7.4}][\text{OCl}^-]}$$

 \Rightarrow

$$[\text{HOCl}] = [\text{OCl}^-]$$

$$\text{Free residual chlorine} = [\text{HOCl}] + [\text{OCl}^-]$$

 \Rightarrow

$$\frac{2 \times 10^{-3}}{2 \times 35} = [\text{HOCl}] + [\text{OCl}^-]$$

 \Rightarrow

$$2.857 \times 10^{-5} = 2[\text{OCl}^-]$$

 \Rightarrow

$$[\text{OCl}^-] = 1.429 \times 10^{-5} \text{ moles/litre}$$

 \Rightarrow

$$[\text{OCl}^-] = 1.429 \times 10^{-5} \times 51 \times 10^3 \text{ mg/l}$$

 \Rightarrow

$$[\text{OCl}^-] = 0.729 \text{ mg/l}$$

16. (d)



| | Test sample | Seeded sample | Diluted sample |
|--|-------------|---------------|----------------|
|--|-------------|---------------|----------------|

| | | | |
|--|------|--------|--------|
| | 5 mL | 295 mL | 300 mL |
|--|------|--------|--------|

| | | | |
|-------------|--------|--------|---|
| Initial DO: | 5 mg/l | 7 mg/L | x |
|-------------|--------|--------|---|

| | | | |
|-----------|---|---|---|
| Final DO: | - | - | y |
|-----------|---|---|---|

$$x = \frac{5 \times 5 + 7 + 295}{300} = 6.967 \text{ mg/l}$$

Also, $x - y = 6.967$ (after 5 days of incubation)

$$\Rightarrow y = 0 \text{ mg/l}$$

\therefore Final DO of diluted sample is zero.

Hence, the readings should be discarded.

17. (c)

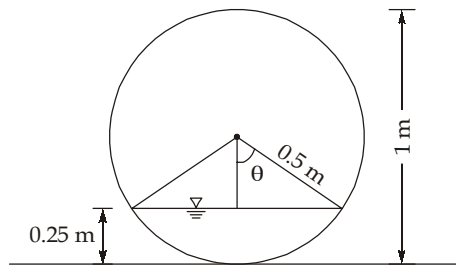
Relative stability for a sample is given by,

$$s = \frac{\text{O}_2 \text{ available in effluent}}{\text{Total O}_2 \text{ required for 1st stage BOD}}$$

$$\begin{aligned} \text{Carbonaceous demand (1st stage BOD)} &= \text{Total oxygen demand} - \text{Nitrogenous demand} \\ &= 110 - 60 = 50 \text{ mg/l} \end{aligned}$$

$$\therefore s = \frac{40}{50} \times 100 = 80\%$$

18. (b)



$$\cos \theta = \frac{0.5 - 0.25}{0.5} = \frac{0.25}{0.5} = \frac{1}{2}$$

$$\Rightarrow \theta = \frac{\pi}{3}$$

$$\text{Cross-sectional area of flow, } A = r^2 \left[\theta - \frac{\sin 2\theta}{2} \right]$$

$$\Rightarrow A = (0.5^2) \left[\frac{\pi}{3} - \left\{ \sin \left(2 \times \frac{\pi}{3} \right) \right\} \frac{1}{2} \right]$$

$$\Rightarrow A = \frac{1}{4} \left[\frac{\pi}{3} - \frac{\sqrt{3}}{4} \right]$$

$$A = 0.1535 \text{ m}^2$$

$$\text{Wetted perimeter of flow} = r (2\theta)$$

$$= 0.5 \times 2 \times \frac{\pi}{3} = 1.047 \text{ m}$$

$$\therefore \text{Hydraulic radius} = \frac{A}{P} = \frac{0.1535}{1.047} = 0.147 \text{ m}$$

19. (c)

Concentration of gas = 2.8 ppm

 $\Rightarrow 10^6 \text{ m}^3 \text{ air holds } 2.8 \text{ m}^3 \text{ gas}$ $\Rightarrow 1 \text{ m}^3 \text{ air holds } 2.8 \times 10^{-6} \text{ m}^3 \text{ gas}$ \therefore Ideal gas law is valid, \therefore

$$PV = nRT$$

$$P = 2 \text{ atm}$$

$$V = 2.8 \times 10^{-6} \text{ m}^3$$

$$T = 294 \text{ K}$$

$$R = 82.05 \times 10^{-6} \text{ atm m}^3/\text{mol. K}$$

$$\therefore 2 \times 2.8 \times 10^{-6} = n \times 82.05 \times 10^{-6} \times 294$$

$$n = 2.32146 \times 10^{-4} \text{ mol}$$

(Note: The concentration is asked in 1 m^3 of air)

$$\therefore n = 2.32146 \times 10^{-4} \times (10^{-2} \times 10^2)$$

$$\Rightarrow n = 2.32146 \times 10^2 \mu \text{ mol}$$

$$\Rightarrow n = 232.146 \mu \text{ mol}$$

 $\Rightarrow 1 \text{ m}^3 \text{ of air has } 232.146 (\simeq 232.15) \mu \text{ mol of gas.}$

20. (d)

| River | Waste water |
|---------------------------------|-----------------------------------|
| $Q_R = 60 \text{ m}^3/\text{s}$ | $Q_w = 6 \text{ m}^3/\text{s}$ |
| $\text{DO} = 7 \text{ mg/l}$ | $\text{DO} = 0 \text{ mg/l}$ |
| $\text{BOD}_5 = 9 \text{ mg/l}$ | $\text{BOD}_5 = 210 \text{ mg/l}$ |

Given data:

$$k_D = 0.5/\text{day}$$

$$k_R = 1/\text{day}$$

$$(\text{DO})_{\text{sat}} = \frac{7}{0.8} = 8.75 \text{ mg/l}$$

$$(\text{DO})_{\text{mix}} = \frac{60 \times 7 + 6 \times 0}{66} = 6.364 \text{ mg/l}$$

$$(\text{BOD}_5)_{\text{mix}} = \frac{60 \times 9 + 6 \times 210}{66} = 27.273 \text{ mg/l}$$

$$\text{Initial DO deficit } (D_0) = 8.75 - 6.364 = 2.386 \text{ mg/l}$$

$$\text{Ultimate BOD } (L_0) = \frac{(\text{BOD}_5)_{\text{mix}}}{1 - e^{-0.5 \times 5}} = \frac{27.273}{1 - e^{-2.5}} = 29.71 \text{ mg/l}$$

Min DO occurs at critical time t_c where,

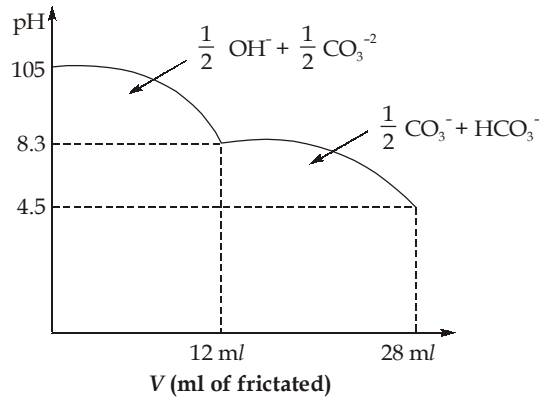
$$t_c = \frac{1}{k_R - k_D} \ln \left[\frac{k_R}{k_D} \left\{ 1 - D_0 \times \left(\frac{k_R - k_D}{k_D L_0} \right) \right\} \right]$$

$$\Rightarrow t_c = \frac{1}{1 - 0.5} \ln \left[\frac{1}{0.5} \left\{ 1 - 2.386 \times \left(\frac{1 - 0.5}{0.5 \times 29.71} \right) \right\} \right]$$

$$\Rightarrow t_c = 1.219 \text{ days}$$

$$\begin{aligned}\therefore \text{Distance of critical DO deficit} &= V \times t_c \\ &= \frac{1 \times 1.219 \times 86400}{1000} \text{ km} = 105.3 \text{ km}\end{aligned}$$

21. (d)



As per graph

12 ml of 0.02 NH_4SO_4 is used to neutralize alkalinity due to $\frac{1}{2}\text{CO}_3^{2-} + \text{OH}^-$

i.e. in given 200 ml water sample, alkalinity due to $\frac{1}{2}\text{CO}_3^{2-} + \text{OH}^-$ is 12 mg

\therefore In 1000 ml of water sample, alkalinity due to $\frac{1}{2}\text{CO}_3^{2-} + \text{OH}^-$ is $\frac{12}{250} \times 1000 = 48 \text{ mg/lit}$

$$\therefore \frac{1}{2}\text{CO}_3^{2-} + \text{OH}^- = 48 \text{ mg/lit as CaCO}_3 \quad \dots(1)$$

Now,

$$\text{pH} = 10.5$$

$$\therefore \text{pOH} = 14 - 10.5 = 3.5$$

$$\text{Now } \text{pOH} = -\log[\text{OH}^-]$$

$$\Rightarrow 3.5 = -\log[\text{OH}^-]$$

$$\Rightarrow [\text{OH}^-] = 10^{-3.5} \text{ mole/lit}$$

$$\therefore \text{No. of mole} = \frac{\text{Weight}}{\text{Molecular weight}}$$

$$\Rightarrow 10^{-3.5} = \frac{\text{Weight}}{17}$$

$$\Rightarrow \text{OH}^- = 17 \times 10^{-3.5} \text{ gm}$$

$$\therefore \text{Number of gm. eq.} = \frac{\text{Given weight}}{\text{Equivalent weight}} = \frac{17 \times 10^{-3.5}}{(17/1)} = 10^{-3.5} \text{ gm eq.}$$

\therefore Alkalinity due to OH^- in terms of $\text{CaCO}_3 = 10^{-3.5} \times 50 \times 1000 = 15.81 \text{ mg/lit. as CaCO}_3$

Put in eq. (1),

$$\text{Alkalinity due to carbonate} = (48 - 15.81) \times 2 = 64.38 \text{ mg/lit. as CaCO}_3$$

22. (b)

$$\begin{aligned}
 Q &= 50000 \text{ m}^3/\text{day} \\
 t_d &= 30 \text{ s} \\
 \text{Volume of tank} &= Q t_d \\
 &= \frac{50000}{86400} \times 30 = 17.361 \text{ m}^3
 \end{aligned}$$

We know

$$P = \mu V G^2$$

 \Rightarrow

$$P = 10^{-3} \times 17.361 \times (800)^2$$

$$P = 11111.04 \text{ Watt} = \frac{11111.04}{746} = 14.894 \text{ hp}$$

\therefore Shaft and motor are not 100% efficient i.e., they are efficient 80% and 90% respectively

$$\therefore P = \frac{14.894}{0.90 \times 0.80} = 20.686 \text{ hp} \simeq 20.7 \text{ hp}$$

23. (c)

$$\text{We know } 1 \mu\text{g}/\text{m}^3 = \frac{1 \text{ ppm} \times \text{molecular mass of pollutant}}{l/\text{mole of pollutant at given temperature and pressure}} \times 10^3 \text{ l}/\text{m}^3 \quad \dots(1)$$

The value of l/mol (i.e. volume, in litres occupied by one molecule of pollutant gas) at 0°C and 1 atm pressure (i.e. 760 mm Hg) is equal to 22.4.

At other value of temperature and pressure, its value is governed by

$$\begin{aligned}
 \text{Avagardor's law as: } \quad \frac{V_1 P_1}{T_1} &= \frac{V_2 P_2}{T_2} \\
 V_1 &= \text{Volume at } 0^\circ\text{C of 1 atm pressure i.e. } 22.4 \text{ l/mol} \\
 P_1 &= 760 \text{ mm Hg} \\
 T_1 &= 0^\circ\text{C} = 273 \text{ K}
 \end{aligned}$$

$$\therefore \frac{22.4 \times 760 \text{ mm of Hg}}{273^\circ\text{K}} = \frac{V_2 \times 760 \text{ mm of Hg}}{(273^\circ + 20^\circ)\text{K}}$$

$$\Rightarrow V_2 = 22.4 \times \frac{293}{273} = 24.04 \text{ l/mol}$$

Put in eq.(1)

$$1 \mu\text{g}/\text{m}^3 = \frac{1 \text{ ppm} \times 64}{24.04} \times 10^3$$

$$\therefore 60 \mu\text{g}/\text{m}^3 = \frac{\text{SO}_2 \text{ in ppm} \times 64 \times 10^3}{24.4}$$

$$\Rightarrow \text{SO}_2 \text{ in ppm} = 0.023 \text{ ppm}$$

24. (b)

$$\text{Treated water} = 25000 \text{ m}^3/\text{day}$$

$$\text{or } 25000 \times 10^3 \text{ lt/day} = 25 \text{ MLD}$$

$$\text{Required chlorines per day} = 10 \text{ kg/day}$$

$$\text{Chlorine required per litre} = \frac{10 \times 10^6}{25 \times 10^6} = 0.4 \text{ mg/lit.}$$

$$\text{The residual Cl}_2 \text{ left} = 0.22 \text{ mg/lit.}$$

$$\text{So, chlorine demand} = 0.4 - 0.22 = 0.18 \text{ mg/lit.}$$

$$\begin{aligned} \text{Chlorine demand per day} &= 0.18 \times 25 \times 10^6 \times 10^{-6} \\ &= 4.5 \text{ kg/day} \end{aligned}$$

25. (d)

Given,

$$L_e = 1.5 \text{ L}$$

$$\frac{L_e}{L} = \frac{1 - \eta}{1 - \eta_e}$$

(η = Porosity)

$$1.5 = \frac{1 - 0.4}{1 - \eta_e}$$

$$\eta_e = 0.6$$

$$V_s = \frac{(G-1)gd^2}{18\nu} = \frac{(2.65-1) \times 981 \times (0.066)^2}{18 \times 1.31 \times 10^{-2}}$$

$$V_s = 29.9 \text{ cm/sec}$$

$$\eta_e = \left(\frac{\bar{V}_s}{V_s} \right)^{0.22}$$

(\bar{V}_s = Backwash velocity)

$$0.6 = \left(\frac{\bar{V}_s}{29.9} \right)^{0.22}$$

$$\bar{V}_s = 2.933 \text{ cm/sec}$$

26. (a)

$$\text{Total mass (solid + water)} = 100 \text{ kg}$$

$$\text{Total moisture} =$$

$$\begin{aligned} &\frac{1}{100} (15 \times 6 + 24 + 6 + 33 \times 5 + 4.2 \times 0.5 + 0.49 \times 2 + 0.13 \times 0.5 \\ &+ 1.18 + 0.5 + 0.35 \times 0.5 + 17.97 + 60 + 1.67 + 6 + 2.01 \times 8) \end{aligned}$$

$$= 15.9739 \text{ kg}$$

$$\text{Mass of solids} = 100 - 15.9739 = 84.0261 \text{ kg}$$

$$\text{Total moisture (on dry basis)} = \frac{\text{Mass of water}}{\text{Mass of solids}} = \frac{15.9739 \times 100}{84.0261} = 19.01\%$$

27. (d)

Sample size (1 ml)

$$\text{Number of positive tubes} = \frac{288}{360} \times 5 = 4$$

Sample size (0.1 ml)

$$\text{Number of positive tubes} = \frac{144}{360} \times 5 = 2$$

Sample size (0.01 ml)

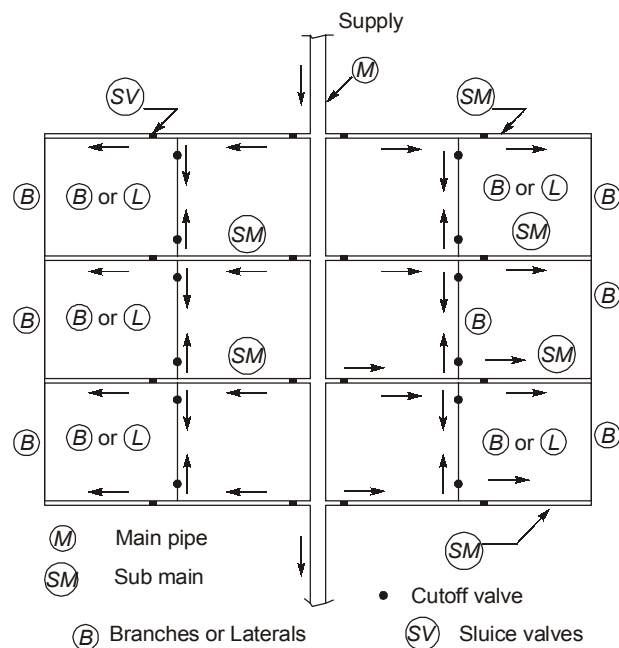
$$\text{Number of positive tubes} = \frac{72}{360} \times 5 = 1$$

Positive combination is 4 - 2 - 1 and negative combination is 1 - 3 - 4.

But dilution of standard used is 10 ml, 1 ml, 0.1 ml.

So, MPN index for negative results = $38 \times 10 = 380$

28. (c)

Statement 2: Sonoscope is used for detection of leakage in underground water mains.**Statement 3:** In grid system, one main pipe runs through centre and branches and laterals run in grid pattern which are interconnected.

29. (b)

Let x g of NaOH be added to solution to change pH from 7 to 9.

$$(\text{pOH})_{\text{initial}} = 14 - 7 = 7$$

$$(\text{pOH})_{\text{final}} = 14 - 9 = 5$$

$$[\text{OH}^-]_{\text{initial}} = 10^{-7} \text{ mol/l.}$$

$$[\text{OH}^-]_{\text{final}} = 10^{-5} \text{ mol/l.}$$

According to question,

For 1 litre of solution

$$\frac{x}{40} + 10^{-7} = 10^{-5} \quad (\text{Molecular weight of NaOH} = 40 \text{ g/mol})$$

$$\Rightarrow \frac{x}{40} = 10^{-5} - 10^{-7} = 99 \times 10^{-7}$$

$$\begin{aligned} \Rightarrow x &= 99 \times 40 \times 10^{-7} \text{ g} \\ x &= 99 \times 4 \mu\text{g} \\ &= 396 \mu\text{g} \end{aligned}$$

∴ 396 μg/l of NaOH is required to change pH from 7 to 9.

30. (a)

| Component | % by mass | % Moisture | Energy (kJ/kg) | Dry (mass(kg)) | Total Energy (kJ) |
|-----------------|-----------|------------|----------------|----------------|-------------------|
| Food waste | 20 | 70 | 4650 | 6 | 93000 |
| Paper | 40 | 6 | 16750 | 37.6 | 670000 |
| Card board | 15 | 5 | 16300 | 14.25 | 244500 |
| Plastics | 7.5 | 2 | 32600 | 7.35 | 244500 |
| Garden trimming | 7.5 | 60 | 6500 | 3 | 48750 |
| Wood | 5 | 20 | 18600 | 4 | 93000 |
| Tin cans | 5 | 3 | 700 | 4.85 | 3500 |
| Total | | | | 77.05 | |

$$\text{Moisture content} = \left(\frac{100 - 77.05}{100} \right) \times 100 = 22.95\%$$

$$\text{Unit energy content} = \frac{\text{Total energy}}{\text{Total weight}} = \frac{1397250}{100} = 13972.50 \text{ kJ/kg}$$

∴ Energy content on ash-free dry basis

$$= 13972.50 \left(\frac{100}{100 - 22.95 - 5} \right) = 19392.78 \text{ kJ/kg}$$

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