

RSSB-JE

2020

Rajasthan Staff Selection Board

Combined Junior Engineer Direct Recruitment Examination

Civil Engineering

Theory of Structures (SOM)

Well Illustrated **Theory** with
Solved Examples and **Practice Questions**



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Theory of Structures (SOM)

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8.1 Introduction

Assumption:

- (i) The material is homogenous, isotropic and elastic in which Hooke's law is valid.
- (ii) The shear stress is constant along the width but very along the depth.

8.2 Shear Stress Distribution in Beams

Let τ be the shear stress in a layer at a distance y from $N.A$ where a particular section is subjected to $S.F = V$

$$\tau = \frac{VA\bar{y}}{Ib}$$

$V = SF$ (Shear force) at the section where shear stress is to be found out

$A\bar{y} =$ Moment of area of section above the level at which shear stress is to be found out

$I =$ Moment of inertia of complete section about NA

$b =$ Width of the section at the level where shear stress is to be found out



NOTE

Shear force per unit length of beam is called shear flow. (q)

$$q = \frac{VA\bar{y}}{I} = \tau.b$$

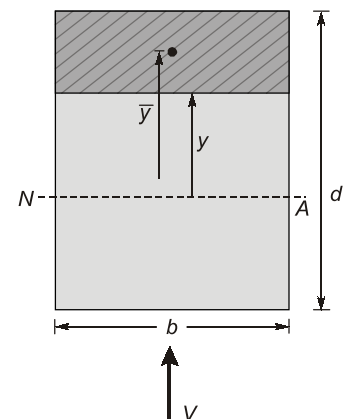
8.3 Shear Stress Distribution in Rectangular Section

$$\tau = \frac{VA\bar{y}}{Ib}$$

$$A\bar{y} = b \left(\frac{d}{2} - y \right) \left[y + \frac{\frac{d}{2} - y}{2} \right]$$

$$= b \left(\frac{d}{2} - y \right) \left(\frac{y + \frac{d}{2}}{2} \right)$$

$$= \frac{b}{2} \left(\frac{d^2}{4} - y^2 \right)$$



⇒ Shear stress,

$$t = \frac{VA\bar{y}}{Ib} = \frac{V \frac{b}{2} \left(\frac{d^2}{4} - y^2 \right)}{\left(\frac{bd^3}{12} \right) \times b}$$

$$\tau = \frac{6V}{bd^3} \cdot \left(\frac{d^2}{4} - y^2 \right)$$

From above equation, it is clear that variation of shear stress is parabolic.

Also,

From above equation:

At $y = \pm d/2$; $\tau = 0$

At $y = 0$ (i.e. at neutral axis)

$$\tau = \tau_{\max}$$

$$\tau_{\max} = \frac{6V}{bd^3} \cdot \frac{d^2}{4} = \frac{3}{2} \cdot \frac{V}{bd} = \frac{3}{2} \tau_{\text{avg}}$$

$$\tau_{\max} = \frac{3}{2} \tau_{\text{avg}}$$

$$\tau_{\max} = 1.5 \tau_{\text{avg}}$$

- Let at a distance y' from N.A where

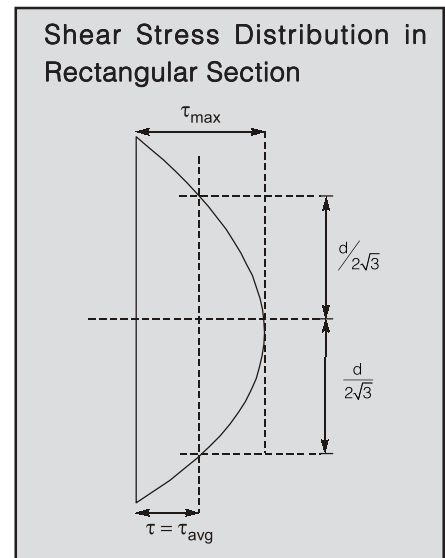
$$\tau_{\text{av}} = \tau$$

$$\frac{V}{bd} = \frac{6V}{bd^3} \cdot \left(\frac{d^2}{4} - y'^2 \right)$$

$$\frac{d^2}{6} = \frac{d^2}{4} - (y')^2$$

$$(y')^2 = \frac{d^2}{4} - \frac{d^2}{6} = \frac{d^2}{12}$$

$$\left(y' = \pm \frac{d}{2\sqrt{3}} \right)$$



⇒ Shear Stress will be equal average shear stress at $\frac{d}{2\sqrt{3}}$ distance from neutral axis.



Example - 8.1 A rectangular beam of width 100 mm is subjected to maximum shear force of 60 kN. The corresponding maximum shear stress in the cross-section is 4 N/mm². What is the depth of beam?

- (a) 150 mm (b) 225 mm (c) 200 mm (d) 100 mm

Ans. (b)

Given

$$\tau_{\max} = 4 \text{ N/mm}^2$$

$$\tau_{\max} = 1.5 \times \tau_{\text{avg}}$$

$$\therefore 4 \frac{N}{\text{mm}^2} = 1.5 \times \frac{60 \times 10^3 N}{100 \text{mm} \times d(\text{mm})}$$

$$\Rightarrow d = 225 \text{ mm}$$



Example-8.2 If a beam of rectangular cross-section is subjected to a vertical shear force S , then how much shear force will be carried by the upper one third of the section?

- (a) zero (b) $\frac{7V}{27}$ (c) $\frac{8V}{27}$ (d) $\frac{V}{3}$

Ans. (b)

S.F resisted by small strips, $dF = \int \tau.(bdy)$

S.F resisted by upper one third portion :

$$F = \int_{\frac{d}{6}}^{\frac{d}{2}} \tau.(bdy) = \int_{\frac{d}{6}}^{\frac{d}{2}} \frac{6V}{bd^3} \left(\frac{d^2}{4} - y^2 \right) .bdy$$

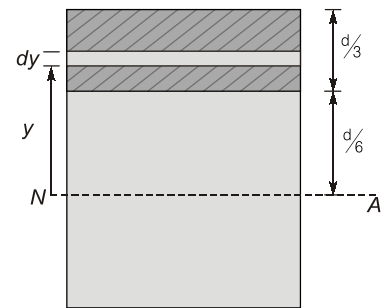
$$= \frac{6V}{d^3} \left[\frac{d^2}{4} \left(\frac{d}{2} - \frac{d}{6} \right) - \frac{1}{3} \left(\frac{d^3}{8} - \frac{d^3}{216} \right) \right]$$

$$= \frac{6V}{d^3} \left[\frac{d^3}{12} - \frac{26d^3}{3 \times 216} \right]$$

$$F = 6V \left[\frac{1}{12} - \frac{26}{648} \right]$$

$$F = \frac{7V}{27}$$

\therefore S.F resisted by top $\frac{1}{3}$ rd portion = $\frac{7V}{27}$



8.4 Shear Stress Distribution in Circular Sections

$$b = EF = 2\sqrt{R^2 - y^2}$$

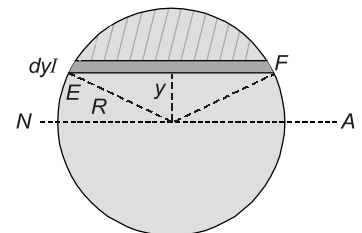
$$A\bar{y} = \int_y^R ydA$$

$$= \int_y^R (bdy) \cdot y.$$

$$= \int_y^R y \left[2\sqrt{R^2 - y^2} \right] dy$$

$$= \frac{2}{3} (R^2 - y^2)^{3/2}$$

$$\tau = \frac{V \frac{2}{3} (R^2 - y^2)^{3/2}}{\left(\frac{\pi R^4}{4} \right) \times 2\sqrt{R^2 - y^2}} = \frac{4V}{3\pi R^4} \times (R^2 - y^2)$$



At $y = \pm R$
 $\tau = 0$

Also, variation of shear stress is parabolic.

At $y = 0$ (i.e. at NA)

$$\tau = \tau_{\max} = \frac{4V}{3\pi R^4} (R^2) = \frac{4V}{3\pi R^2} = \frac{4}{3} \cdot \frac{V}{\pi R^2} = \frac{4}{3} \tau_{\text{avg}}$$

$$\tau_{\max} = \frac{4}{3} \tau_{\text{avg}}$$

Let at a distance y' from NA (Neutral axis) Where.

$$\tau_{\text{avg}} = \tau$$

$$\frac{V}{\pi R^2} = \frac{4V}{3\pi R^4} (R^2 - y'^2)$$

$$\frac{4V}{3R^4} (R^2 - y'^2) = 1$$

$$4R^2 - 4y'^2 = 3R^2$$

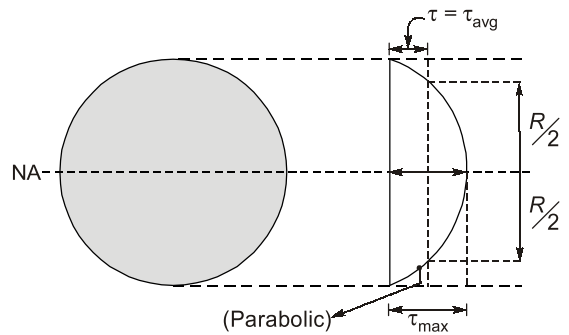
$$R^2 = 4y'^2$$

$$y'^2 = \frac{R^2}{4}$$

$$y' = \pm \frac{R}{2}$$

Shear Stress Distribution

1. $\tau_{\max} = \frac{4}{3} \tau_{\text{avg}}$
2. τ_{\max} occur at $y = 0$ i.e. at NA.
3. τ_{avg} occur at $y = \frac{R}{2}$ from NA.



NOTE

For Hollow circular section,

R_1 = Internal radius

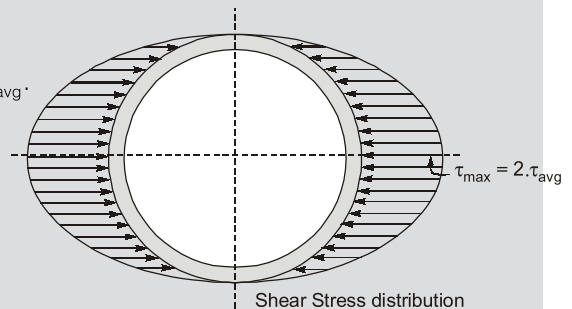
R_2 = Outer radius.

$$\tau_{\max} = \frac{4}{3} \left(\frac{R_1^2 + R_1 R_2 + R_2^2}{R_1^2 + R_2^2} \right) \cdot \tau_{\text{avg}}$$

For thin circular tube ($R_1 \approx R_2$)

$$\tau_{\max} = \frac{4}{3} \left(\frac{3R_1^2}{2R_1^2} \right) \cdot \tau_{\text{avg}}$$

$$\tau_{\max} = 2 \cdot \tau_{\text{avg}}$$



8.5 Shear Stress Distribution in Triangular Section

Let τ be the shear stress at a distance y from vertex.

$$EF = b'$$

$$A\bar{y} = \left(\frac{1}{2} b' y \right) \left(\frac{2}{3} h - \frac{2y}{3} \right)$$

$$\tau = \frac{VA\bar{y}}{Ib}$$

$$= \frac{V\left(\frac{1}{2}b'y\right)\left\{\frac{2}{3}(h-y)\right\}}{\left(\frac{bh^3}{36}\right)b'}$$

$$\tau = \frac{12V}{bh^3}(hy = y^2)$$

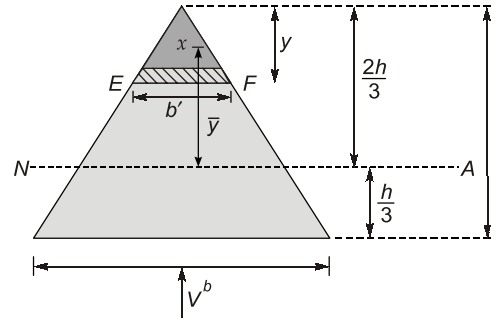


Fig. 9.6

From above equation, it is clear that variation is parabolic

Also,

At $y = 0 ; \tau = 0$

$y = h ; \tau = 0$

for τ to be maximum

$$\frac{d\tau}{dy} = 0$$

$$h - 2y = 0$$

$$y = \frac{h}{2}$$

∴

$$\tau_{\max} = \frac{12V}{bh^3} \cdot \left(\frac{h^2}{2} - \frac{h^2}{4}\right) = \frac{3V}{bh} = \frac{3}{2} \times \frac{V}{\frac{1}{2}bh} = \frac{3}{2} \tau_{\text{avg}}$$

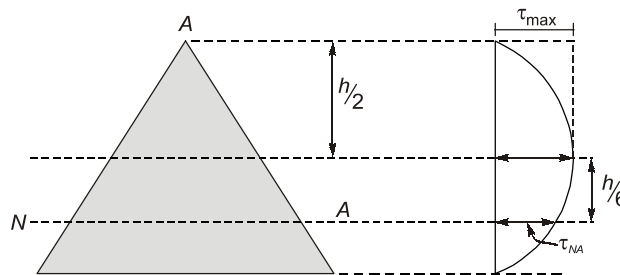
$$\tau_{\max} = 1.5 \tau_{\text{avg}}$$

$$\tau_{NA} = \frac{12V}{bh^3} \left(\frac{2h^2}{3} - \frac{4h^2}{9}\right) = \frac{12V}{bh^3} \left(\frac{6h^2 - 4h^2}{9}\right)$$

$$= \frac{4}{3} \cdot \frac{V}{bh} \times 2 = \frac{4}{3} \times \frac{V}{\frac{1}{2}bh}$$

$$\tau_{NA} = \frac{4}{3} \tau_{\text{avg}}$$

Shear Stress distribution



$$\tau_{\max} = 1.5 \tau_{\text{avg}} \text{ (at } y = h/2 \text{ from vertex)}$$

$$\tau_{NA} = \frac{4}{3} \tau_{\text{avg}}$$

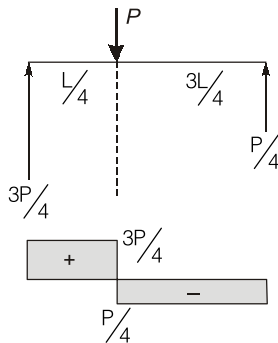
Distance between NA and τ_{\max} location = $\frac{h}{6}$

ANSWER KEY // **STUDENT'S ASSIGNMENTS**

1. (c) 2. (b) 3. (b) 4. (c) 5. (d)
6. (c) 7. (c) 8. (a) 9. (c) 10. (b)
11. (c) 12. (a) 13. (c) 14. (a) 15. (a)

HINTS & SOLUTIONS // **STUDENT'S ASSIGNMENTS**

2. (b)



maximum shear force in the beam = $\frac{3P}{4}$

$$\begin{aligned} \therefore \tau_{\max} &= \frac{3}{2} \cdot \tau_{\text{avg}} \\ &= \frac{3}{2} \cdot \left(\frac{3P/4}{bh} \right) = \frac{9P}{8bh} \end{aligned}$$

4. (c)

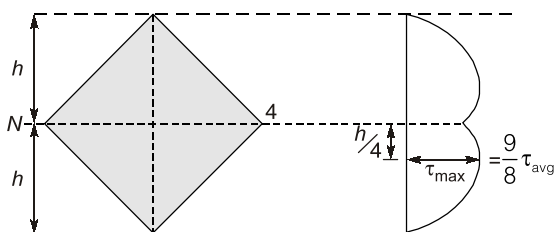
$$\tau_{\text{NA}} = \frac{4}{3} \cdot \tau_{\text{avg}} = \frac{4}{3} \cdot \frac{F}{\left(\frac{1}{2}bh\right)} = \frac{8F}{3bh}$$

10. (c)

$$\frac{(\tau_{\text{web}})_{\text{at junction}}}{(\tau_{\text{flange}})_{\text{at junction}}} = \frac{B}{b} = \frac{100}{20} = 5$$

$$(\tau_{\text{web}})_{\text{at junction}} = 5 \times 12 = 60 \text{ MPa}$$

11. (c)



$$2h = B\sqrt{2} \Rightarrow h = \frac{B\sqrt{2}}{2}$$

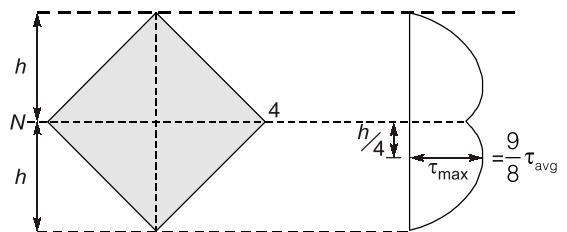
τ_{\max} occur at $\frac{h}{4}$ from NA

$$= \frac{B\sqrt{2}}{2 \times 4} = \frac{B\sqrt{2}}{2 \times 4} \times \frac{\sqrt{2}}{\sqrt{2}} = \frac{B}{4\sqrt{2}}$$

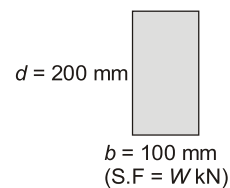
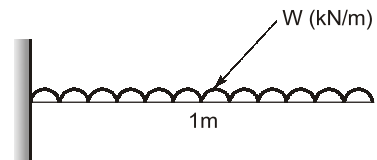
13. (c)

$$\tau_{\max} = \frac{3}{2} \cdot \tau_{\text{avg}} = 1.5 \times \frac{200 \times 10^3}{200 \times 300} = 5 \text{ MPa}$$

14. (a)



15. (a)



$$d = 200$$

$$\tau_{\max} = 1.5 \text{ N/mm}^2$$

$$1.5 = \frac{3}{2} \times \frac{(W \times 1) \times 10^3}{100 \times 200}$$

$$W = \frac{400 \times 100 \times 1.5}{3 \times 10^3} = 20 \text{ kN/m}$$

■■■■