Civil Engineering

Environmental Engg: Vol-II

(Sewage Disposal & Air Pollution Engineering)

Comprehensive Theory

with Solved Examples and Practice Questions





MADE EASY Publications Pvt. Ltd.

Corporate Office: 44-A/4, Kalu Sarai (Near Hauz Khas Metro Station), New Delhi-110016

E-mail: infomep@madeeasy.in Contact: 011-45124660, 8860378007 Visit us at: www.madeeasypublications.org

Environmental Engineering: Vol-II (Sewage Disposal & Air Pollution Engineering)

© Copyright by MADE EASY Publications Pvt. Ltd.

All rights are reserved. No part of this publication may be reproduced, stored in or introduced into a retrieval system, or transmitted in any form or by any means (electronic, mechanical, photo-copying, recording or otherwise), without the prior written permission of the above mentioned publisher of this book.

First Edition: 2015
Second Edition: 2016
Third Edition: 2017
Fourth Edition: 2018
Fifth Edition: 2019
Sixth Edition: 2020
Seventh Edition: 2021
Eighth Edition: 2022

© All rights reserved by MADE EASY PUBLICATIONS Pvt. Ltd. No part of this book may be reproduced or utilized in any form without the written permission from the publisher.

Contents

Environmental Engg: Vol-II

(Sewage Disposal & Air Pollution Engineering)

Ch	apter 1		
Wa	nste Water Characteristics1	3.2.2 Sedimentation	
1.1	Characteristics of Sewage2	3.2.3 Sunlight	
	1.1.1 Physical Characteristics of Sewage	3.2.4 Oxidation	
	and their Testing2	3.2.5 Reduction	37
	1.1.2 Chemical Characteristic3	3.3 Various Factors on which Natural Forces of	
1.2	Population Equivalent10	Purification Depends	
1.3	Relative Stability12	3.3.1 Temperature	
1.4	Ratios	3.3.2 Turbulence	
1.7	1.4.1 BOD/COD Ratio13	3.4 Zone of Pollution in River Stream	
	1.4.2 COD/TOC Ratio	3.5 Indices of Self Purification	39
	Objective Brain Teasers	3.6 Oxygen Deficit of a Polluted River Stream	40
	Objective brain reasers16	3.6.1 Reaeration	40
		3.6.2 Rate of Oxygen Removed	40
Ch	apter 2	3.6.3 The Oxygen Sag Curve	41
Bio	ochemical Reactions in Treatment	3.7 Disposal of Waste Waters in Lakes and	
of\	Waste Water23	Management of Lake Water	46
2.1	Biochemical Reactions23	3.8 Stratification of Lakes	46
2.2	Biological Growth24	3.9 Biological Zones in Lakes	46
2.3	Decomposition of Sewage26	3.10 Productivity of Lake	47
	2.3.1 Aerobic Decomposition26	3.11 Disposal of Waste Water in Sea Water	48
	2.3.2 Anaerobic Decomposition26	Objective Brain Teasers	56
2.4	Various Types of Bacteria31		
	Objective Brain Teasers32	Chapter 4	
		Design of Sewerage System	
Ch	apter 3	and Sewer Appurtenances	60
Dis	sposing of the Sewage Effluents33	4.1 Difference in the Design of	
3.1	Disposal by Dilution33	Water Supply Pipes and Sewer Pipes	60
	3.1.1 Condition Favouring Disposal by Dilution33	4.2 Sewer Materials	61
	3.1.2 Standards of Dilution for Discharge of	4.3 Laying of Sewer Pipes	67
	Wastewaters into Rivers33	4.3.1 Marking of the Alignment	67
3.2	Dilution in Rivers and Self Purification of	4.3.2 Excavation of Trench	68
	Natural Streams34	4.3.3 Bracing of the Trench	68
	3.2.1 Dilution and Dispersion35	4.3.4 Dewatering of Trench	68

	4.3.5 Laying of Pipes	68	5.2	Primar	y Treatments	115
	4.3.6 Testing of the Sewer Pipes	69		5.2.1	Screening	. 119
4.4	Types of Collection System	69		5.2.2	Comminutors	. 121
	4.4.1 Sanitary Sewer	69		5.2.3	Disposal of Screenings	. 122
	4.4.2 Storm Sewer	70		5.2.4	Grit Removal Basins	. 123
	4.4.3 Combined Sewer System	71		5.2.5	Design Criteria	. 123
4.5	Assumptions in Sewer Design	72		5.2.7	Grit Chambers or Grit Channels	. 128
4.6	Design Data	73		5.2.8	Detritus Tanks	. 132
4.7	Maximum Velocity	74		5.2.9	Tanks for Removing Oils and Grease	. 132
4.8	Equal Degree of Self cleansing	78		5.2.10	Sedimentation	. 134
	4.8.1 Shield's Expression for Self-cleansing Velocity.	78		5.2.11	Types of Settling	. 135
4.9	Egg Shaped Sewer	80	5.3	Secon	dary Treatment of Sewage	. 137
4.10	Storm Water Drainage	81		5.3.1	Secondary Treatment Through	
	4.10.1 Computing the Peak Drainage				Biological Filtration of Sewage	. 137
	Discharge by the Use of Rational Formula	81		5.3.2	Contact Beds for Biological	
	4.10.2 Rational Formula	83			Filtration of Sewage	. 138
4.11	Sewer Appurtenances	85		5.3.3	Intermittent Sand Filters for	
	4.11.1 Manholes	85			Biological Filtration of Sewage	. 139
	4.11.2 Drop Manhole	86		5.3.4	Trickling Filters for Biological	
	4.11.3 Lamp Holes	87			Filtration of Sewage :	
	4.11.4 Clean-Outs	87			(Attached Growth System)	. 139
	4.11.5 Street Inlets or Gullies	88	5.4	Secon	dary Sedimentation	. 149
	4.11.6 Catch Basins or Catch Pits	88		5.4.1	Secondary Settling Tanks or Humus Tanks.	. 149
	4.11.7 Flushing Tanks	89	5.5	Digest	ion and Disposal of Primary	
	4.11.8 Grease and Oil Traps	89		and Se	condary Sludge	. 151
	4.11.9 Inverted Siphons	90		5.5.1	Sludge Sources, Characteristics	
	4.11.10 Storm Water Regulators or				and Quantities	. 151
	Storm Relief Works	92		5.5.2	Sludge Treatment Processes	. 151
4.12	Shape of Sewer Pipes	94		5.5.3	Constituents	. 153
4.13	Corrosion of Concrete Sewers			5.5.4	Sludge Digestion Process	. 155
	Due to Biological Reaction	95		5.5.5	Stages of the Sludge Digestion Process	. 157
4.14	Methods of Ventilation	96		5.5.6	Factors affecting Sludge Digestion	
4.15	Maintenance of Sewer	96			and their Control	. 158
4.16	Cleaning of Sewers	96	5.6	Sludge	Digestion Tank or Digesters	
	4.16.1 Precautions to be Taken While Entering Sewer	s.97		(Aerob	ic Suspended Culture)	. 160
	Objective Brain Teasers	. 104		5.6.1	Constructional Details	. 160
				5.6.2	Design Consideration	. 160
Ch	apter 5			5.6.3	High Rate Digesters	. 161
	•	10		5.6.4	Two Stage Sludge Digesters	. 162
	atment of Sewage1		5.7	Dewat	ering Disposal of Digested Sludge	. 165
5.1	Treatment Methods			5.7.1	Objective and Process Selection	. 165
	5.1.1 Unit Operations	. 110 . 110		5.7.2	Disposal of Dewatered Sludge	. 167
	TIZ UIII FIOLESS	LIU				

5.8	Second	dary Treatment Through Activated		6.1.1	Municipal Solid Wastes	250
	Sludge	Process (Aerobic Suspended Culture) 170		6.1.2	Industrial Wastes	251
	5.8.1	Various operations and Units of		6.1.3	Municipal Solid Waste Composition	
		an Activated Sludge Plant170			and Quality of Generated	
	5.8.2	Secondary Sedimentation Tank of			Municipal Solid Wastes	252
		an Activated Sludge Plant174	6.2	Funct	ional Elements of Solid Waste Management	253
	5.8.3	Sludge Thickner and Sludge Digestion		6.2.1	Factors affecting Generation Rates	254
		Tank of an Active Sludge Plant 176		6.2.2	On-site Handling, Storage and Processing	254
	5.8.4	Bulking and Foaming Sludge in		6.2.3	Collection of Solid Wastes	255
		an Activated Sludge Treatment Plant 178		6.2.4	Transfer and Transport	257
	5.8.5	Sludge Stabilization179		6.2.5	Processing Techniques	257
	5.8.6	Chlorine Stabilization180	6.3	Dispo	sal of Municipal Solid Wastes (MSW)	258
	5.8.7	Design Consideration Involved in an		6.3.1	Disposal of Refuse by Open Dumping	259
		Activated Sludge Plant180		6.3.2	Disposal of Refuse by Land Filling	259
	5.8.8	Modification of the Basic Activated		6.3.3	Disposal of Refuse by Shredding	
		Sludge Process186			and Pulverisation	267
	5.8.9	Advantages Offered by an Activated		6.3.4	Disposal of Refuse by Composting	268
		Sludge Plant 190		6.3.5	Disposal of Refuse by Incineration	
	5.8.10	Disadvantages Offered by an			and Thermal Pyrolysis	271
		Activated Sludge Plant191		6.3.6	Disposal of Refuse by Barging it	
5.9	Second	dary Treatment Through Rotating			out into the Sea	273
	Biological Contactors193		6.4	Storage, Transportation and		
5.10	Oxidat	ion Ditches or Extended Aeration Lagoons 194		Dispo	sal of Industrial Solid Wastes	273
	5.10.1	Continuous Flow Type 195		Objec	tive Brain Teasers	275
	5.10.2	Intermittent Flow Type 195				
	5.10.3	Reaction Kinetics 195	Ch	apte	er 7	
5.11	Aerobi	c Stabilizaton Units197		-	ution	277
	5.11.1	Oxidation Ponds and Stabilization Ponds 197				
	5.11.2	Design Parameter for Oxidation Pond 198	7.1		e and Classification of Air Pollutants	
	5.11.3	Use of Lagoons for Disposal of Raw Sludge202		7.1.1	Air Contaminants	
	5.11.4	Aerated Lagoons203	7.2		Natural Contaminants	
5.12	Anaero	obic Stabilisation Units 204	7.2		muand Casandam. Air Dallistanta	
	5.12.1	Anaerobic and Facultative	7.3		ry and Secondary Air Pollutants	
		Stabilisation Ponds204	7.4		s of Air Pollution	
	5.12.2	Septic Tanks204		7.4.1	Effects of Air Pollution on Human Health	
	5.12.3	Imhoff Tank 212		7.4.2	Effects of Air Pollution on Plants	283
	5.12.4	High Rate Anaerobic Systems213		7.4.3	Effects of Air-pollutants on	202
		Objective Brain Teasers234		7.4.4	Live Stock Animals	283
				7.4.4		
Ch:	aptei	r 6		-	and Services	
	•			7.4.5	Effects of Air Pollutant on Aquatic Life	284
		ste Management250		7.4.6		
6.1	Types c	of Solid Wastes250			and Regional Environment	284

7.5	Photochemical Air Pollution284	8.	1.6 Effect of Noise on Wildlife	329
	7.5.1 Nitrogen Dioxide Photochemical Reaction 284	8.2 Cł	naracteristics of Sound and its Measurement	329
	7.5.2 Sulphur Dioxide Photochemical Reaction 285	8.	2.1 Levels of Noise	330
	7.5.3 Aldehyde Photochemical Reaction 285	8.	2.2 Sound Intensity Level	33
7.6	Photochemical Smog285	8.	2.3 Octave Band Analysis	332
7.7	Composition and Structure of the Atmosphere 286	8.	2.4 Averaging Sound Pressure Levels	332
7.8	Acid Rain	8.3 No	oise Rating Systems	33
7.9	Global Warming287	8.	3.1 The LN Concept	333
7.10	Ozone Layer Depletion 288	8.	3.2 The Leq Concept	333
7.11	Meteorology and Natural Purification Process 289	8.4 Sc	ources of Noise and Their Noise Levels	334
7.12	Elemental Properties of the Atmosphere289	8.	4.1 Traffic Noise	334
7.13	Control of Air Pollution291	8.	4.2 Industrial Noise	334
	7.13.1 Natural Self Cleansing Properties of	8.	4.3 Noise Produced by Other Sources	33
	the Environment291	8.5 No	oise Abatement and Control	33
	7.13.2 Engineered System292	O	bjective Brain Teasers	336
7.14	Control Devices for Particulates292			
7.15	Control Devices for Gaseous Pollutants301	Char	oter 9	
	7.15.1 Adsorption Unit301	_	lation of Buildings for	
	7.15.2 Absorption Units302			226
	7.15.3 Condensation Units305		rolling Indoor Air Pollution	
	7.15.4 Combustion Unit305		fects of Occupancy of a Space	
7.16	Automotive Emission Control308		urpose of Ventilation	
7.17	Dispersion of Air Pollutants into the Atmosphere 308	•	rstems of Ventilation	
7.18	Negative Lapse Rate and Inversion 310		3.1 Natural Ventilation	
	7.18.1 Radiation Inversion311		3.2 Mechanical Ventilation	34
	7.18.2 Subsidence Inversion311		ınctions and Types of Traps being	
7.19	Impact of Winds on Dispersion of Pollutants 311		sed in Sanitary Plumbing System	
7.20	Plume Behaviour312	-	stem of Plumbing	
7.21	Design of Stack Height315		5.1 Two Pipe System	
	Objective Brain Teasers319	9.	5.2 One Pipe System	
			5.3 Single Stack System	349
Ch	apter 8	9.	5.4 Partially Ventilated Single Stack or	
	ise Pollution328		Single Pipe System	
		9.6 Sa	nitary Fitting and Other Accessories	
8.1	The Effects of Noise	9.	6.1 Water Closets (W.C)	35
	8.1.1 Noise induced Annoyance328	9.	6.2 Urinals	
	8.1.2 Noise induced Disease		6.3 A Flushing Cistern	
	8.1.3 Sleeplessness	9.7 Ve	entilation of House Drains	35
	8.1.4 Communication Interference		ntisiphonage Pipes	
	8.1.5 Noise induced Hearing Loss329	0	bjective Brain Teasers	35

Waste Water Characteristics

INTRODUCTION

- When untreated sewage is discharged into river stream, floating solids present in the discharged sewage get decompose and create foul smells and bad odours.
- The organic matter present in the discharged sewage consume the dissolved oxygen of river stream in getting oxidised and decrease the dissolved oxygen causing fish kills and other undesirable effects.
- Waste water are usually classified as industrial waste water or municipal waste water.
- Industrial waste water with characteristic compatible with municipal waste water is often discharged into the municipal sewers.
- Many industrial waste water require pretreatment to remove non-compatible substances prior to discharge into the municipal system.
- Water collected in municipal waste water system contains wide variety of contaminants.
- Commonly found contaminants with their source and environmental significance are given below.

Table: 1.1 Important waste water contaminants					
SL. No	c. Contaminant	Source	Environmental Significance		
1	Industrial wastes		Cause sludge deposits and anaerobic condition in aquatic environment		
2			Cause biological degradation		
3	Pathogens Domestic water		Transmit communicable diseases		
4	Nutrients	Domestic and Industrial waste	Cause eutrophication		
5	Refractory Organics Industrial waste		Cause taste and odour problems		

Hence for its safe disposal, it is very necessary to study the characteristics and behavior of sewage.





NOTE: Even though municipal sewage is 99.9 percent water, it requires treatment, if nuisance is to be avoided.

1.1 Characteristics of Sewage

The quality of sewage can be checked and analysed by studying and testing its physical, chemical and bacteriological (biological) characteristics.

1.1.1 Physical Characteristics of Sewage and their Testing

The most important physical characteristics of waste water are

- (i) Turbidity
- (ii) Colour
- (iii) Odour
- (iv) Temperature

(i) Turbidity

- Sewage is normally turbid, resembling dirty dish water or waste water from baths having other floating matter like faecal matter, pieces of paper, cigarette-ends, match-sticks, greases, vegetable debris, fruit skins, soaps, etc. The turbidity increases as sewage becomes stronger.
- Turbidity is measured photometrically by determining the percentage of light of a given intensity that is either absorbed or scattered.
- It is expressed as the amount of suspended solids in mg/L or ppm (parts per million).
- The degree of turbidity can be measured and tested by
 - (i) Turbidity rods

(ii) Turbidimeters



- (i) Turbidity rod consists of an aluminium rod which is graduated, as to give the turbidity in silica units
- (ii) Turbidimeter works on the principle of measuring the interference caused by water sample to the passage of light rays.

(ii) Colour

- The colour of sewage can normally be detected by the naked eye and it indicates the freshness of sewage.
- If colour is yellow, grey or light brown, it indicates fresh sewage and if the colour is black or dark brown it indicates stale or septic sewage.
- Some industrial waste water may also add colours to the domestic waste water.
- The common method of colour removal is by coagulation followed by sedimentation.

NOTE: When all the oxygen has disappeared from sewage, it becomes septic

(iii) Odour

- Fresh sewage is practically odourless.
- Odour in waste water usually is caused by gases produced by the decomposition of organic matter.
- The most characteristic odour of stale or septic waste water is that of hydrogen sulphide which is produced by anaerobic microorganisms that reduces sulphate to sulphides.
- The extent of odour present in a particular sample of waste water is measured by a term called odour intensity, which is related with the Threshold Odour Number (TON).

NOTE: Threshold odour number, represents the number of dilutions required to reduce an odour.









(iv) Temperature

- The temperature of waste water is commonly higher than that of water supply because of addition of warm waste from households and industrial activities.
- The average temperature of sewage in India is 20°C, which is near about the ideal temperature for the biological activities.
- The temperature of water is very important parameter because of its effect on the aquatic life, the chemical reactions and reaction rates and suitability of the water for beneficial uses.

1.1.2 Chemical Characteristic

Chemical characteristics are result of the solvent properties of water and they are often important in specifying water quantity. Important chemical characteristics of waste water are listed below:

- (a) Total solids, suspended solids and settleable solids (b) pH value
- (c) Chloride content
- (e) Presence of fats, greases and oils
- (g) Dissolved oxygen
- (i) Theoretical oxygen demand (ThOD)
- (k) Bio-chemical oxygen demand (BOD)

- (d) Nitrogen content
- (f) Sulphides, sulphates and H₂S gas
- (h) Chemical oxygen demand (COD)
- (j) Total organic carbon

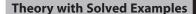
(a) Total Solids, Suspended Solids and Settleable Solids

- Solids present in sewage may be in any of the four forms, suspended solids, dissolved solids, colloidal solids and settleable solids.
- Suspended solids are those solids which remain floating in sewage.
- Colloidal solids are finely divided solids remaining either in solution or in suspension.
- Settleable solids are that portion of solid matter which settles out, if the waste water is allowed to remain undisturbed for a period of 2 hours.

NOTE: It has been estimated that about 1000 kg of sewage contains about 0.45 kg of total solids, out of which 0.225 kg is in solution, 0.112 kg is in suspension and 0.112 kg is settleable.

- The solids in sewage comprise of both: the organic and inorganic solids, which is about 45 and 55 percent of total solids respectively.
- Inorganic matter consist of sand, gravel, debris, chlorides, sulphates etc.
- Organic matter consist of
 - Carbohydrates such as cellulose, cotton, fibre, sugar etc.
 - Fats and oils from kitchen, garages, shops etc.
 - Nitrogenous compounds like proteins, urea, fatty acids etc.
- The amounts of various kinds of solids present in waste water can be determined as follows:
 - (a) Total amount of solids can be determined by evaporating a known volume of waste water sample and weighing the dry residue left. The mass of residue left divided by the volume of sample is total solids in mg/l.
 - (b) The suspended solids also called non-filtuable solids, as they are retained by a filter of 1 μ m pores. Their quantity can be determined by passing a known volume of sewage sample through glass-fibre filter apparatus and weighing the dry residue left and dividing by volume of sample filtered will give suspended solids in mg/l.





(c) The difference between the total solids (S_1) and the suspended solids (S_2) represent dissolved solids plus colloidals or filterable solids (S_3)

 $S_3 = S_1 - S_2$. i.e.

(d) Now the total suspended solids (S_2) may either be volatile or fixed. Let the volatile suspended solids concentration be $S_{A}(mg/l)$.

 $S_2 - S_4 = S_5$ will represent the fixed solids

(e) The quantity of settleable solids can be determined using Imhoff cone. Waste water is allowed to stand in the cone for two hours and the quantity of solids settled down in the bottom is directly read out, which gives an approximate amount of settleable solids.

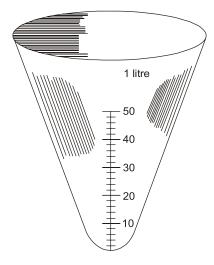


Fig. 1.1 Imhoff cone (Conical Glass Vessel)

(b) pH Value

The pH value of sewage indicates the negative log of hydrogen ions concentration present in sewage.

 $pH = -\log H^{+} \text{ or } H^{+} = (10)^{-pH}$ i.e.

- The determination of pH value is very important, as it gives an idea about certain treatments which depends upon pH value.
- The pH value can be measured by the help of potentiometer which measure the electrical potential exerted by the hydrogen ions, and thus indicating their concentration.

NOTE: The fresh sewage is generally alkaline in nature (with pH more the 7) but as time passes its pH tends to fall due to production of acids by bacterial action in anaerobic or nitrification processes.

Chloride Content (c)

- Chlorides are generally present in municipal sewage and are derived from the kitchen wastes, human faeces and urinary discharge etc.
- The normal chloride content of domestic sewage is 120 mg/l, however, large amount of chlorides may enter from industries like ice-cream plants, meat salting etc. thus increasing the chloride content of waste water.
- The chloride content can be measured by titrating the waste water with standard silver nitrate solution, using potassium chromate as an indicator.

(d) **Nitrogen Content**

The presence of nitrogen in sewage indicates the presence of organic matter and may occur in one or more of the following forms:

- (a) Free ammonia called ammonia nitrogen (indicates recent pollution).
- (b) Albuminoid nitrogen called organic nitrogen (indicates quantity of nitrogen before decomposition has started).
- (c) Nitrites (indicates partly decomposed condition).
- (d) Nitrates [indicates old pollution (fully oxidised)]







NOTE: Lack of nitrates causes the body to turn bluish, it may lead the child to turn blue. Hence, this disease, popularly called blue baby disease or mathemoglobinemia.

(e) Presence of Fats, Oils and Greases

- Greases, fats and oils are derived in sewage from the discharge of animals and vegetable matter or from industries like garages, kitchen of hotels and restaurants etc.
- Such matter form scum on the top of the sedimentation tanks and clog the voids of the filtering media. Therefore, they interfere with the normal treatment methods, and hence need proper detection and removal.
- Fats and oils are compounds of alcohol or glycerol with fatty acids.
- The amount of fats and greases in sewage sample can be determined by evaporating it and then mixing the residual solids left, with ether (hexane). The solution is then poured off and evaporated, leaving behind the fats and greases as residue, which can be easily weighed.

(f) Sulphides, Sulphates and Hydrogen Sulphide Gas

- Sulphides and sulphates are formed due to the decomposition of various sulphur containing substances present in sewage.
- This decomposition also leads to evolution of hydrogen sulphide gas, causing bad smells and odours, besides causing corrosion of concrete sewer pipes.
- The aerobic and facultative bacteria, oxidise the sulphur and its compounds present in sewage to
 initially form sulphides, which ultimately breakdown to form sulphate ions (SO₄⁻), which is a stable
 and unobjectionable end product.

(g) Dissolved Oxygen (D.O)

- Dissolved oxygen present in sewage is very important for respiration of aerobic micro-organism as well as for all other aerobic life forms.
- The dissolved oxygen in fresh sewage depends upon temperature. If the temperature of sewage is more, the D.O. content will be less. Maximum quantity of D.O. that can remain mixed in water at a particular temperature is called Saturation Dissolved Oxygen.
- The D.O. content of sewage is generally determined by the Winkler's Method.

NOTE: The solubility of oxygen in sewage is 95% of that in distilled water.

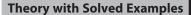


- 1. It is necessary to ensure at least 4 ppm of dissolved oxygen in discharged treated sewage, otherwise fish are likely to be killed.
- 2. Dissolved oxygen is desirable in drinking water but it is usually removed from boiler feed waters because of the manner in which it accelerates corrosion of metals.

(h) Chemical Oxygen Demand (COD)

- The COD test is used to measure the content of organic matter of waste water, both biodegradable and non biodegradable.
- The oxygen equivalent of organic matter that can be oxidised is measured by using a strong chemical
 oxidising agent in an acidic medium. [Potassium dichromate has been found to be excellent for this
 purpose. (K₂Cr₂O₇)]
- This test is also called dichromate-oxygen demand test







(i) Theoretical Oxygen Demand (ThOD)

The oxygen required to oxidise the organic matter present in a given waste water can be theoretically computed, if the organics present in waste water are known. Thus, if the chemical formulas and the concentration of the chemical compounds present in water are known, we can easily calculate the ThOD of each of these compounds by writing the balanced reaction for the compound with oxygen to produce CO₂, H₂O and oxidised inorganic components.

Remember: For most practical cases COD = ThOD (taken) [However, generally ThOD ≥ COD ≥ BOD > TOC]

(j) Total Organic Carbon

It is another important method of expressing organic matter in terms of its carbon content.

(k) Bio-Chemical Oxygen Demand

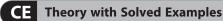
The organic matter is of two types:

- (a) Biologically active or biologically degradable (b) Biologically inactive
- Biochemical oxygen demand is used as a measure of the quantity of oxygen required for oxidation of biodegradable organic matter present in water sample by aerobic biological action.
- Oxygen demand of waste water is exerted by three classes of materials
 - (a) Carbonaceous organic materials.
 - (b) Oxidisable nitrogen derived from nitrite, ammonia and other organic nitrogen compounds which serves as food for specific bacteria (Nitrosomonas and nitrobacter).
 - (c) Chemical reducing compounds e.g. Fe²⁺, SO_3^{2-} (sulfites), S^{2-} (sulfide) which are oxidised by dissolved oxygen.
- For domestic sewage, nearly all oxygen demand is due to carbonaceous organic material and is determined by BOD test.



When the nitrogeneous matter is also to be removed in treatment process, Nitrogeneous demand is also found out. This is called N-BOD (i.e. O_2 required for conversion of nitrogeneous matter to nitrate). Removal of nitrogen from the system is achieved by 1st oxidising the nitrogeneous organic matter to nitrate (i.e. nitrification) and then denitrifying the nitrate to release nitrogen gas (N_2) which goes out of the waste water.

- BOD test constitutes the most important test for waste waters and is discussed below:
 - (i) If sufficient oxygen is available in waste water, the useful aerobic bacteria will flourish and cause the aerobic biological decomposition of waste water, which will continue until oxidation is completed. The amount of oxygen consumed in this process is the BOD.
 - (ii) Hence the BOD of water during 5 Days at 20°C is generally taken as the standard and is about 68% of the total demand. A 10 days BOD is about 90% of the total.
 - (iii) This standard 5 day BOD, is written as BOD₅ or simply as BOD and is determined in the laboratory by mixing or diluting a known volume of a sample of waste water with known volume of aerated pure water and then calculating the D.O. of this diluted sample. The diluted sample is then incubated for 5 days at 20°C. The D.O. of diluted sample after this period of incubation, is again calculated. The difference between the initial D.O. value and the final D.O. value will







indicate the oxygen consumed by the sewage sample in 5 days. The BOD in ppm is then calculated by using the equation.

BOD or BOD₅ =
$$\frac{\text{D.O. consumed in the}}{\text{test by the diluted sample}} \times \left[\frac{\text{volume of the diluted sample}}{\text{volume of the undiluted sewage sample}} \right]$$

The factor in the bracket is called dilution factor

NOTE: Sample is diluted with dilution water so that sufficient oxygen is available during the incubation period of 5 days

- The first demand occurs due to the oxidation of organic matter and is called carbonaceous demand or first stage demand or initial demand and the later demand occurs due to biological oxidation of ammonia, and is called Nitrogeneous demand or second stage demand. The term BOD is usually used to mean the first state BOD i.e. the demand due to the presence of Carbonaceous matter alone.
- Nitrogenous demand starts only after 5-8 days because the reproduction rate of nitrofication bacteria is slow.

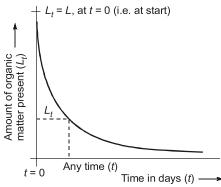


Fig. 1.2 Ist Stage BOD curve



Nitrification bacteria are autotrophs. They derive carbon for their growth from CO_2 . Hence they have to spend energy in reducing ' CO_2 ' to C. Thus energy available for their reproduction is less, thereby growth rate is less. Carbonaceous matter oxidising bacteria are heterotrophs. They derive carbon from organic matter directly. Hence energy is not spent in getting carbon, thereby their reproduction rate is more.

Reaction Kinetics

At a certain temperature, the rate of deoxygenation is assumed to be directly proportional to the amount of organic matter present in sewage at that time. i.e.

$$\frac{dL_t}{dt} = -kL_t \qquad \dots (i)$$

where L_t = oxygen equivalent of carbonaceous oxidisable organic matter present in sewage after t days from the start of oxidation in mg/l.

t = time in days

k = rate constant signifying the rate of oxidation of organic matter and it depends upon the nature of organic matter and temperature. Its unit is per day.

Integrating equation (i)

$$\int \frac{dL_t}{L_t} = \int -kdt$$

or $\log_e L_t = -kt + C \qquad \dots (ii)$

when t = zero i.e. at start $L_t = L$. Substituting in equation



or
$$C = \log_e L$$

$$\therefore \text{ From equation (ii)}$$

$$\log_e L_t = -kt + \log_e L$$
or
$$\log_e L_t - \log_e L = -kt$$
or
$$\log_e \frac{L_t}{L} = -kt$$
or
$$2.3 \log_{10} \frac{L_t}{L} = -kt$$
or
$$\log_{10} \frac{L_t}{L} = -kt$$

using 0.434 $k = K_D$, where K_D is the Deoxygenation rate constant or more strictly, the BOD rate constant (on base 10) at the given temperature

$$\therefore \log_{10} \frac{L_t}{L} = -K_D t$$
or
$$\boxed{L_t = L(10)^{-K_D \cdot t}} \quad ...(iii)$$

Now, L is the organic matter present at the start of BOD reaction and L_t is the organic matter left after t days (Fig. 1.3)

:. BOD =
$$L - L_t = L - L(10)^{-K_D \cdot t}$$

$$(BOD)_t = L[1 - 10^{-K_D \cdot t}] \quad ... (iv)$$

The value of K_D determines the speed of the BOD reaction, without influencing the ultimate BOD. It is found to vary with temperature of sewage and this relationship is approximately given by the equation.

$$K_{D(T^{\circ})} = K_{D(20^{\circ})} [1.047]^{T-20^{\circ}} ...(v)$$

 $K_{D(20^{\circ}\text{C})}$ = Deoxygenation constant at 25°C. It numerical value varies between 0.05 to 0.2 per day, depending upon the nature of the organic matter present in sewage.

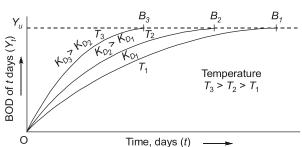


Fig. 1.3 BOD exertion as a function of K_D

Various Types of Waters and Waste water			
Water Type	K _D value per day		
Tap waters	< 0.05		
Surface waters	0.05 – 0.1		
Municipal wastewaters	0.1 – 0.15		
Treated sewage effluents	0.05 – 0.1		

Table: 1.2 Typical Values of K_D at 20°C for

Simple compounds such as sugars and starches are easily utilised by the micro-organism and have a high K_D rate, while complex molecules such as phenols are difficult to assimilate and hence have low K_D values. Some typical K_D values are given in Table 1.2.

 $K_{D(T^{\circ})}$ = Deoxygenation constant at temperature T°C.

- Equation (v) shows that K_D will be higher at higher temperature, which means that the speed at which BOD is consumed in the oxidation of the organic matters is higher at higher temperature. This mean that the entire carbonaceous organic matter will get oxidised quickly and in lesser time at higher temperature.
- Equation (iv) is called the first stage equation of BOD reaction and is presented by the curve OAB in figure 1.4 below:



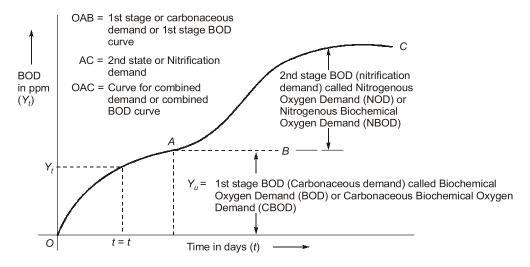


Fig. 1.4 Different BOD stages with time

• The portion AC of the curve represent the nitrification stage which follows the carbonaceous stage, so that the BOD curve for the complete oxidation is represented by OAC.



- (i) The equations given is only for the first stage BOD and have nothing to do with the second stage BOD.
- (ii) Values of 5-days 20°C BOD of municipal waste water generally vary between 100 to 500 mg/l.

Estimation of K_D

$$K_D = 2.61 \frac{A}{B}$$

where, A =Slope of line

B = Intercept of the line on y-axis

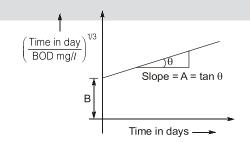


Fig. 1.5 Relation between K_D and Time (in days)

Example 1.1 For a waste, the 5-day BOD at 20°C is found to be 200 mg/L. For the same waste, 5-day BOD at 30°C will be

- (a) less than 200 mg/L
- (b) more than 200 mg/L

(c) 200 mg/L

(d) zero, as the bacteria cannot withstand such a high temperature

Ans. (b)

The effect of temperature on BOD can be approximately given by the Van't Hoff-Arrhenius models:

At
$$K_T = K_{20}(1.047)^{T-20}$$

 $T = 30^{\circ}\text{C}, K_T > K_{20}$
 $BOD_{5,30^{\circ}\text{C}} = L_0 \left(1 - e^{-K_T t}\right)$
 $BOD_{5,30^{\circ}\text{C}} > BOD_{5,20^{\circ}\text{C}}$



Example 1.2 The ultimate BOD value of a waste

- (a) increases with temperature
- (b) decreases with temperature
- (c) remains the same at all temperatures
- (d) doubles with every 10°C rise in temperature

Ans. (c)

Ultimate BOD represents the biodegradable organic matter, so it will remain unchanged.

Example 1.3 For a waste water the BOD_5 at 20° is found to be 200 mg/I. For same waste BOD_3 at 30°C will be? The reaction constant 'K' (to the base e) is 0.2 per day.

Solution:

$$\begin{split} \text{BOD}_5 & \text{ at } 20^{\circ}\text{C} = L_0[1 - e^{-K \times 5}] \\ \text{BOD}_3 & \text{ at } 30^{\circ}\text{C} = L_0[1 - e^{-K' \times 3}] \\ L_0 &= \text{BOD}_u \\ 200 &= L_0[1 - e^{-5K}] \\ \text{BOD}_3 &= L_0[1 - e^{-3K'}] \end{split} \tag{Remain constant}$$

From (i) and (ii)

$$\frac{200}{1 - e^{-5K}} = \frac{BOD_3}{1 - e^{-3K'}} \Rightarrow \frac{1 - e^{-3K'}}{1 - e^{-5K}} = \frac{BOD_3}{200}$$
But,
$$K' = K(1.047)^{30-20} = 0.2[1.047]^{10} = 0.316$$

$$\Rightarrow BOD_3 = 200 \left[\frac{1 - 2.72^{-3 \times 0.316}}{1 - 2.72^{-5 \times 0.2}} \right] = \frac{0.6127}{0.632} \times 200 = 193.89 \text{ mg/l}$$

1.2 Population Equivalent

- Average standard BOD of domestic sewage is 80 gms per person per day.
- The number of person which produce the amount of BOD at the rate of 80 gms per person per day
 equal to that produced by industrial sewage is called population equivalent of industrial sewage.
- Industrial wastewater are generally compared with per capita normal domestic wastewater, so as to
 rationally charge the industries for the population caused by them. The strength of the industrial
 sewage is, thus worked out as below.

$$\begin{bmatrix} \text{Standard BOD (5 days)} \\ \text{of industrial sewage} \end{bmatrix} = \begin{bmatrix} \text{Standard BOD (5 days) of domestic} \\ \text{sewage per person per day} \end{bmatrix} \times \begin{bmatrix} \text{Population} \\ \text{equivalant} \end{bmatrix}$$

- The average standard BOD₅ of domestic sewage is worked out to be about 0.08 kg/day/person. Assume, BOD₅ of sewage coming from on industries is worked out as 300 kg/day, then
 - $\Rightarrow \text{ The population equivalent} = \frac{\text{Total BOD}_5 \text{ of the industry in kg/day}}{0.08 \text{ kg/day/person}} = \frac{300}{0.08} = 3750$

Example 1.4 Data from an unseeded domestic waste water BOD test are: 5 m/ of waste in 300 m/ bottle, initial D.O. of 7.8 mg/l, and 5 days DO equal to 4.3 mg/l. Compute

- (a) the BOD; and
- (b) the ultimate BOD, assuming a k_D of 0.10 day⁻¹



Solution:

(a) Initial D.O. = 7.8 mg/l

D.O. after 5 days of incubation = 4.3 mg/l

.. D.O. consumed in 5 days = 7.8 - 4.3 = 3.5 mg/l

 BOD_5 of wastewater = D.O consumed by diluted sample× $\left[\frac{\text{vol. of diluted sample}}{\text{vol. of undiluted sewage used}}\right]$

=
$$3.5 \text{ mg/}l \times \left[\frac{300 \text{ m}l}{5 \text{ m}l}\right] = 210 \text{ mg/}l$$

(b) Now, using equation
$$Y_t = L(1-(10)^{-K_D \cdot t})$$
, we have

$$Y_5 = L(1-(10)^{-K_D.5})$$
 where, $K_D = 0.1$ per day and $Y_5 = 210 \text{ mg/}l$

$$\therefore 210 = L \left[1 - (10)^{-0.1 \times 5} \right] = L \left[1 - \frac{1}{(10)^{0.5}} \right] = L[1 - 0.316] = 0.684 L$$

or
$$L = \frac{210}{0.684} \text{mg/}l = 307.1 \text{ mg/}l$$

Example 1.5 A certain waste has a BOD of 162 mg/L and its flow is 1000 cubic metres per day. If the domestic sewage has a BOD of 80 gram per capita per day, then the population equivalent of the waste would be

(a) 20.25

(b) 1296

(c) 2025

(d) 12960

Ans: (c)

Daily BOD contributed by waste =
$$162 \times \frac{1000 \times 1000}{10^3} = 162,000 \text{ g/day}$$

Population equivalent =
$$\frac{162,000}{80}$$
 = 2025 persons

NOTE: The population equivalent, thus, indicates the strength of the industrial wastewater for estimating the treatment required at the municipal treatment plant

Example 1.6 A dairy processing about 1,33,000 kg of milk daily produces an average of 246 cubic metre per day of waste water with a BOD of 1400 mg/*l*. The principal operations are bottling of milk, and making ice cream, with limited production of cheese. Compute the waste water flow and BOD per 1000 kg of milk received, and the equivalent population of the daily waste discharge.

Solution:

Daily milk processed = 1,33,000 kg

Daily wastewater produced = 246 m³

BOD of wastewater = $1400 \,\text{mg/l}$

Evidently,

Wastewater produced per 1000 kg of milk = $\frac{246}{133000} \times 1000 \text{ m}^3 = 1.85 \text{ m}^3$

$$K_{D(15)} = 0.1[1.047]^{15-20^{\circ}}$$

$$= 0.1(1.047)^{-5}$$

$$= \frac{0.1}{(1.047)^{5}} = 0.079$$

Now, using

$$Y_{t \text{ at } T} = L \left[1 - (10)^{-K_D \cdot t} \right]$$
, we have
 $Y_{3 \text{ at } 15} = 150$

$$= L \left[1 - (10)^{-0.079 \times 3} \right]$$

$$= L \left[1 - \frac{1}{(10)^{0.237}} \right] = 0.422 L$$

or

$$L = 355.53 \,\text{mg/}l$$

$$K_{D(10^{\circ}\text{C})} = 0.1[1.047]^{10-20} = 0.063$$

 $K_{D(15^{\circ}\text{C})} = 0.1[1.047]^{25-20} = 0.1258$

$$K_{D(20^{\circ}\text{C})} = 0.1[1.047]^{30-20} = 0.1583$$

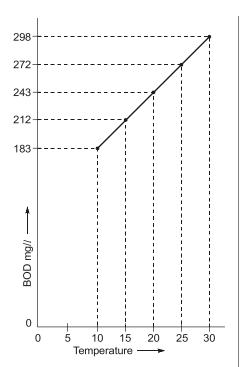
(i)
$$Y_{5 \text{ at } 10^{\circ}\text{C}} = 355.33[1 - (10)^{-0.063 \times 5}] = 183 \text{ mg/}l$$

(ii)
$$Y_{5 \text{ at } 15^{\circ}\text{C}} = 355.33[1 - (10)^{-0.079 \times 5}] = 212 \text{ mg/}l$$

(iii)
$$Y_{5 \text{ at } 20^{\circ}\text{C}} = 355.33[1 - (10)^{-0.1 \times 5}] = 243 \text{ mg/}l$$

(iv)
$$Y_{5 \text{ at } 25^{\circ}\text{C}} = 355.33[1 - (10)^{-0.1258 \times 5}] = 272 \text{ mg/}l$$

(v)
$$Y_{5 \text{ at } 30^{\circ}\text{C}} = 355.33[1 - (10)^{-0.1583 \times 5}] = 298 \text{ mg/}l$$



These calculated five number BOD values w.r.t. temperature are plotted in figure, so as to obtain the requisite graph, which is almost a straight line.



Important Expressions

1. BOD or BOD₅ = D.O consumed in the test by the diluted sample $\times \left[\frac{\text{Volume of the diluted sample}}{\text{Volume of the undiluted sewage sample}} \right]$

2. $(BOD)_t = L \left[1 - 10^{-K_D \cdot t} \right]$ $\Rightarrow \qquad \qquad L_t = L \left[10^{-K_D t} \right]$

 K_D = Deoxygenation constant

L =Organic matter present at the start of BOD reaction

 L_t = Organic matter left after t days

3.
$$K_{D(T^{\circ}C)} = K_{D(20^{\circ}C)} [1.047]^{T-20^{\circ}C}$$

4.
$$K_D = 2.61 \frac{A}{B}$$

where,

where, $A = \text{Slope of line of (time/BOD)}^{1/3} \text{ Vs time)}$

B = Intercept of the line on Y-axis

5. Relative stability, $S = 100 \left[1 - (0.794)^{t_{20}} \right]$

$$S = 100 \left[1 - (0.630)^{t_{37}} \right]$$

6. Population Equivalent = $\frac{\text{Total BOD}_5 \text{ of the industry in kg/day}}{0.08 \text{ kg/day/person}}$



- The most obnoxious odour of waste water is that of hydrogen sulphide, which is produced by anaerobic microorganisms that reduce, sulphates to sulphites.
- The average temperature of sewage in India is 20°C, which is near about the ideal temperature for the biological activities.
- The quantity of settleable solids can be determined using Imhoff cone.
- The pH value can be measured by the help of potentiometer which measure the electrical potential exerted by the hydrogen ions, and thus indicating their concentrations.
- The chloride content can be measured by titrating the sample of waste water with standard silver nitrate solution, using potassium chromate as an indicator.
- Free ammonia indicates recent pollution, nitrates indicates partly decomposed condition and nitrates indicates old pollution.
- Dissolved oxygen less than 4 ppm is detrimental to the survival of fish.
- For most practical cases, COD = ThOD
 However, generally COD > ThOD > BOD > TOC
- Average standard BOD of domestic sewage is 80 gms per person per day.



Objective Brain Teasers

Q.1 An industrial waste water enters a stream having a BOD concentration of 10 mg/L and a flow of 20 m³/s. If the flow of wastewater is 1.5 m³/s and its BOD concentration is 250 mg/L, then the BOD concentration in the stream at a point downstream of the point of confluence of wastewater with the stream will be

- (a) 2.67 mg/L
- (b) 12.09 mg/L
- (c) 13.00 mg/L
- (d) 26.74 mg/L

Q.2 The following data pertain to a sewage sample: Initial DO = 10 mg/L

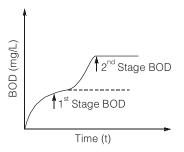
Final DO = 2 mg/L

Dilution to 1%

The BOD of the given sewage sample is

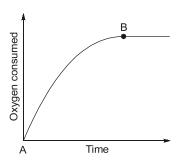
- (a) 8 mg/L
- (b) 10 mg/L
- (c) 100 mg/L
- (d) 800 mg/L

Q.3 The second stage BOD as shown in the figure is due to





- (a) experimental error
- (b) increased activity of bacteria
- (c) nitrification demand
- (d) interference by certain chemical reactions
- Q.4 If the BOD₃ of a wastewater sample is 75 mg/L and reaction rate constant k (base e) is 0.345 per day, the amount of BOD remaining in the given sample after 10 days is
 - (a) 3.12 mg/L
- (b) 3.45 mg/L
- (c) 3.69 mg/L
- (d) 3.92 mg/L
- Q.5 Which one of the following pairs is not correctly matched?
 - (a) BOD/COD = 0 : Waste-water is toxic
 - (b) BOD/COD ≤ 0.2 : Acclimatization of seed is necessary
 - (c) BOD/COD ≥ 0.6 : Waste-water is nonbiodegradable
 - (d) BOD = COD = 0 : Waste-water is devoid of organic matter
- Q.6 What is 5 days 20°C BOD equal to?
 - (a) 3 days 27°C BOD
 - (b) 4 days 30°C BOD
 - (c) 6 days 32°C BOD
 - (d) 7 days 35°C BOD
- Q.7 In context of water polluted with sewage, what does BOD signify?
 - (a) Biological oxygen demand
 - (b) Bacteriological oxygen demand
 - (c) Biochemical oxygen demand
 - (d) Biology of degradation
- Q.8 The figure below shows, BOD curve when the experiment was conducted at 20°C. If the experiment is conducted at 30°C, then the portion AB of the curve



- (a) shifts to the left
- (b) shifts to the right
- (c) remains unchanged
- (d) shrinks
- Q.9 In aerobic environment, nitrosomonas convert
 - (a) NH_3 to NO_2
- (b) NO_2 to NO_3
- (c) NH_3 to N_2O^-
- (d) NO_2 to HNO_3
- Q.10 A sample of sewage is estimated to have a 5 day 20° C BOD of 250 mg/l. If the test temperature be 30°C, in how many days will the same value of BOD be obtained?
 - (a) 1.5 days
- (b) 2.5 days
- (c) 3.3 days
- (d) 7.5 days
- Q.11 Consider the following statements:

The time of BOD assimilation in a stream can be affected by

- 1. Ratio of stream depth to flow width.
- 2. Stream BOD value.
- 3. BOD rate constant.

Which of these statements are correct?

- (a) 1, 2 and 3
- (b) 1 and 2 only
- (c) 2 and 3 only
- (d) 1 and 3 only
- Q.12 Statement (I): The BOD test is conducted for 5 days at 20°C.

Statement (II): The amount of oxygen utilized by microorganisms anaerobically is called BOD.

- (a) Both Statement (I) and Statement (II) are individually true and Statement (II) is the correct explanation of Statement (I)
- (b) Both Statement (I) and Statement (II) are individually true but Statement (II) is NOT the correct explanation of Statement (I)
- (c) Statement (I) is true but Statement (II) is false
- (d) Statement (I) is false but Statement (II) is
- Q.13 High COD to BOD ratio of an organic pollutant represents
 - (a) high biodegradability of the pollutant
 - (b) low biodegradability of the pollutant
 - (c) presence of free oxygen for aerobic decomposition
 - (d) presence of toxic material in the pollutant

Q.20

- Q.14 Biochemical oxygen demand (BOD) of wastewater is a measure of
 - (a) total concentration of biochemicals
 - (b) total concentration of organic matter
 - (c) concentration of biodegradable organic matter
 - (d) concentration of chemically oxidizable matter
- Q.15 Chemical Oxygen Demand (COD) of a sample is always greater than Biochemical Oxygen Demand (BOD) since it represents
 - (a) biodegradable organic matter only
 - (b) biodegradable and non-biodegradable organic matter
 - (c) non-biodegradable organic matter
 - (d) inorganic matter
- Q.16 A waste water sample diluted to 100 times with aeration water had an initial dissolved oxygen (DO) of 7.0 mg/L and after 5 days of incubation at 20°C, the DO was zero. The BOD of waste water is
 - (a) 700 mg/L
 - (b) 100 mg/L
 - (c) Cannot be determined
 - (d) 7 mg/L
- Q.17 A student began an experiment of 5 day 20°C BOD on Monday. Since the 5th day fell on Saturday. The final DO reading was taken on Monday. On calculation BOD (i.e. 7 day 20°C) was found to be 150 mg/L. What would be the 5 day, 20°C BOD (in mg/L). Assume value of BOD rate constant (*K*) at standard temperature of 20°C as 0.23/day (base *e*).
- Q.18 A single rapid test to determine the pollution status of river water is
 - (a) biochemical oxygen demand
 - (b) chemical oxygen demand
 - (c) total organic solids
 - (d) dissolved oxygen
- **Q.19** The 5-day BOD of a wastewater sample is obtained as 190 mg/L (with $k = 0.01 \ h^{-1}$). The ultimate oxygen demand (mg/L) of the sample will be

- (a) 3800
- (b) 475
- (c) 271
- (d) 190
- A portion of waste water sample was subjected to standard BOD test (5 days, 20°C), yielding a value of 180 mg/L. The reaction rate constant (to the base 'e') at 20°C was taken as 0.18 per day. The reaction rate constant at other temperature may be estimated by $k_T = k_{20} (1.047)^{T-20}$. The temperature at which the other portion of the sample should be tested, to exert the same BOD in 2.5 days, is
- (a) 4.9°C
- (b) 24.9°C
- (c) 31.7°C
- (d) 35.0°C
- Q.21 List-I contains some properties of water/waste water and List-II contains list of some tests on water/ waste water. Match List-I with List-II and select the correct answer using the codes given below the lists:

List-I

- A. Suspended solids concentration
- B. Metabolism of biodegradable organics
- C. Bacterial concentration
- D. Coagulant dose

List-II

- 1. BOD
- 2. MPN
- 3. Jar test
- 4. Turbidity

Codes:

	Α	В	C	D
(a)	2	1	4	3
(b)	4	1	2	3
(c)	2	4	1	3
(d)	4	2	1	3

Q.22 To determine the BOD₅ of a waste water sample, 5, 10 and 50 mL aliquots of the waste water were diluted to 300 mL and incubated at 20°C in BOD bottles for 5 days. The results were as follows:

S.No.	Waste-water volume, mL	Initial DO, mg/L	DO after 5 days, mg/L
1.	5	9.2	6.9
2.	10	9.1	4.4
3.	50	8.4	0.0



Based on the data, the average BOD₅ of the waste water is equal to

- (a) 139.5 mg/L
- (b) 126.5 mg/L
- (c) 109.8 mg/L
- (d) 72.2 mg/L

Answers

- 1. (d)
- 2. (d)
- 3. (c)
- 4. (c)
- 5. (c)

- 6. (a) 7. (c)
- 8. (a)
 - 9. (a) 10. (c)
- 11. (c) 12. (c) 13. (b) 14. (c) 15. (b)
- 16. (c)
- 17. 128 18. (d) 19. (c) 20. (d)
- 21. (b) 22. (a)

Hints and Explanations:

Ans.1 (d)

$$BOD_{stream} = \frac{Q_1 y_1 + Q_2 y_2}{Q_1 + Q_2}$$
$$= \frac{20 \times 10 + 1.5 \times 250}{20 + 1.5}$$
$$= 26.74 \text{ mg/L}$$

Ans.2 (d)

BOD = [Initial DO - Final DO] × Dilution Factor Where Dilution Factor

= Volume of diluted sample Volume of undiluted sample

$$= (10-2) \times \frac{100}{1} = 800 \text{ mg/L}$$

Ans.4 (c)

The BOD at any instant is given by

$$L_t = L(1 - e^{-kt})$$

$$\Rightarrow 75 = L(1 - e^{-0.345 \times 3})$$

$$\Rightarrow$$
 $L = \frac{75}{1 - 0.355} = 116.32 \text{ mg/L}$

BOD after 10 days,

$$L_{10} = L(1 - e^{-0.345 \times 10})$$

= 116.32 × 0.968 = 112.63 mg/L

Amount of BOD remaining = $L - L_{10}$

$$= 116.32 - 112.63 = 3.69 \text{ mg/L}$$

Ans.5 (c)

 $BOD_5/COD \ge 0.6$ means waste water is biodegradable

Ans.6 (a)

$$BOD_t = L_0 (1 - e^{-kt})$$

and
$$k_T = k_{20} (1.047)^{T-20}$$

BOD₅ at 20°C will be equal to a BOD_t for given temperature when $k_T t$ is equal to $k_{20} \times 5$.

$$k_T t = k_{20} \times 5$$

or
$$\frac{k_T}{k_{20}} = \frac{5}{t} = (1.047)^{T-20}$$

$$t = \frac{5}{(1.047)^{\mathsf{T}-20}}$$

Τ t(davs)

27°C 3.63

30°C 3.16

32°C 2.88

35°C 2.51

There is no correct answer but (a) is closest.

Ans.8

The BOD of water at any time t is expressed

$$y_t = L_0 (1 - e^{-kt})$$

The value of k is temperature dependent. Because micro-organisms are more active at higher temperatures, the value of k increases with increasing temperatures. The value of y_t approaches L_0 asymptotically indicating that the total, or ultimate, BOD (y_{ij}) is equal to the initial oxygen equivalent of the water L_0 . Thus, at higher temperatures the value of y_t approaches to ultimate BOD earlier, shifting point B on the curve towards the left.

Ans.10(c)

$$BOD_{5, 20^{\circ}C} = L(1 - 10^{-K_D \times t})$$

The effect of temperature on BOD can be given

$$K_{D_{T \circ C}} = K_{D_{20 \circ C}} (1.047)^{T-20}$$

$$\therefore K_{D_{30^{\circ}C}} = K_{D_{20^{\circ}C}} (1.047)^{10}$$

$$K_{D_{30^{\circ}C}} = 1.583 K_{D_{20^{\circ}C}}$$

$$\therefore L\left(1 - 10^{-K_D \times 5}\right) \, = \, L\left(1 - 10^{-1.583 \times K_D \times t}\right)$$

$$\Rightarrow 10^{-K_D \times 5} = 10^{-1.583 \times K_D \times t}$$

$$t = \frac{5}{1.583} = 3.16 \text{ days}$$